

STORM DRAIN DESIGN

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STORM DRAIN DESIGN

I. General

- A. The Sonoma County Water Agency performs drainage review for the City of Cloverdale. Drainage calculations shall be submitted to the Water Agency for review at the same time improvement plans are submitted to the City for review.
- B. Sonoma County Water Agency "Flood Control Design Criteria" shall be used for the design of all drainage improvements within the City of Cloverdale.
- C. These standards are minimum standards and do not preclude the use of a higher standard.

II. Waterways Defined

- A. A "waterway" is defined as being a natural or artificial channel or depression in the surface of the earth or an underground conduit system which provides a course for water flowing as a consequence of storm water runoff.
- B. Major Waterways - having a tributary drainage area of four (4) square miles or more; shall require a design frequency of reoccurrence of once in 100 years.
- C. Secondary Waterways - having a tributary drainage area of between one (1) and four (4) square miles; shall require a design frequency of reoccurrence of once in 25 years.
- D. Minor Waterways - having a tributary drainage area less than one (1) square mile; shall require a design frequency of reoccurrence of once in 10 years.

III. Entrance and Exit Losses

Entrance and exit losses shall be calculated using the loss coefficients in Plate No. 1.

IV. Submittal Requirements

- A. Assumptions used in preparing calculations shall be listed.
- B. The design aids and references which are used in support of the calculations for design of drainage improvements shall be listed. Supply the Water Agency with copies of reference data. If computers are used, the input and output shall be sufficient to allow easy checking.
- C. Hydrology map(s) shall be provided for both on and off-site drainage areas. The maps shall be of sufficient scale and detail to show drainage areas. Drainage areas shall be numbered and outlined to facilitate checking. The area of each drainage area shall be

shown on the map.

- D. Hydrologic and hydraulic calculations showing beginning hydraulic gradeline, energy losses at junctions, bends, structures, friction slopes, etc. shall be submitted.
- E. In addition to the calculations, the hydraulic gradeline, and the energy gradeline shall be shown for all open or closed drainage improvements except gutters.
- F. Plan views, profiles, cross-sections, and details of all drainage facilities including a lot grading plan showing how each lot will drain shall be submitted.
- G. Entrance capacity and gutter depth calculations shall be submitted for all drainage inlets.
- H. Additional information may be required as determined by the Water Agency or the City Engineer.

PLATE 1 - BOX WIDTH IN FEET

PIPE DIAMETER	Kx/Ke	1.0'	1.5'	2.0'	2.5'	3.0'	4.0'	5.0'	6.0'
	8"-12"	0.23/0.16	0.59/0.41	0.76/0.53	0.84/0.59	0.89/0.62	0.94/0.65	0.96/0.67	0.97/0.68
15"	0.03/0.02	0.40/0.23	0.63/0.44	0.76/0.53	0.83/0.58	0.90/0.63	0.94/0.63	0.96/0.67	
18"		0.23/0.16	0.50/0.35	0.66/0.46	0.76/0.53	0.86/0.60	0.91/0.64	0.94/0.65	
24"			0.23/0.16	0.44/0.31	0.59/0.41	0.76/0.59	0.84/0.59	0.89/0.62	
30"				0.23/0.16	0.40/0.28	0.63/0.44	0.76/0.53	0.83/0.58	
36"				0.06/0.04	0.23/0.16	0.50/0.35	0.66/0.46	0.76/0.53	
42"					0.08/0.06	0.36/0.25	0.55/0.39	0.68/0.47	
48"						0.23/0.16	0.44/0.31	0.59/0.41	
54"						0.11/0.08	0.33/0.23	0.50/0.35	
60"						0.03/0.02	0.23/0.16	0.40/0.28	
66"							0.13/0.09	0.13/0.22	
72"							0.06/0.04	0.23/0.16	

Definitions:

Kx = Exit Loss coefficient
Exit loss for an outlet into a creek = 1.0

Ke = Entrance Loss coefficient
Loss coefficients are to be applied to the velocity head to determine the minor loss.

V. Materials

- A. Storm sewer main lines shall be Class I, Class II, or Class III RCP or HDPE pipe as required by the Engineer and shown on the approved plans.
- B. Storm drain laterals shall be PVC, SDR35, HDPE or RCP as approved by the Engineer.
- C. Minimum diameter for all storm sewer main lines shall be 18".

VI. Manholes

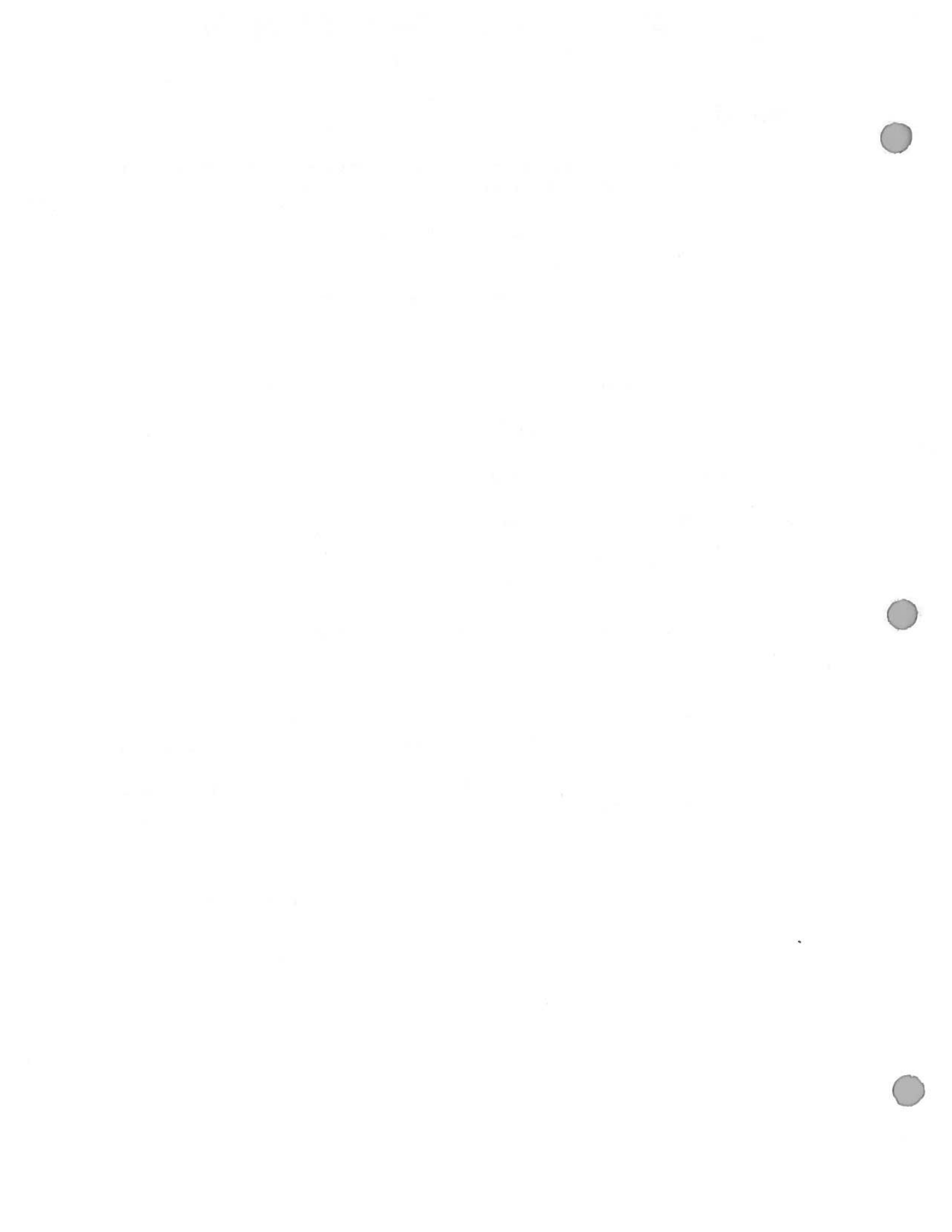
- A. A manhole is required at every horizontal or vertical change in alignment.
- B. Maximum distance between manholes is 300 feet.
- C. A manhole is required at the end of every main.
- D. Minimize the number of manholes.
- E. 60" diameter manholes are required for mains larger than 36" in diameter, or deeper than 7' (indicate manhole diameter on plans).
- F. When pipe size increases, set inlet crown at least as high as the outlet crown.

VII. Cover

- A. Minimum cover for all storm sewer lines is 24" from top of pipe to finished grade, or 12" from top of pipe to bottom of structural section, whichever is more restrictive.
- B. If conditions do not allow for minimum cover, storm sewer line shall be capped or pipe material shall be changed per the direction of the Engineer.

VIII. Testing and Acceptance

- A. All storm sewer lines shall be cleaned of construction debris and sediment before final inspection.
- B. Prior to acceptance by the City, all storm sewer lines shall be video taped as required in this Standard Construction Specification.

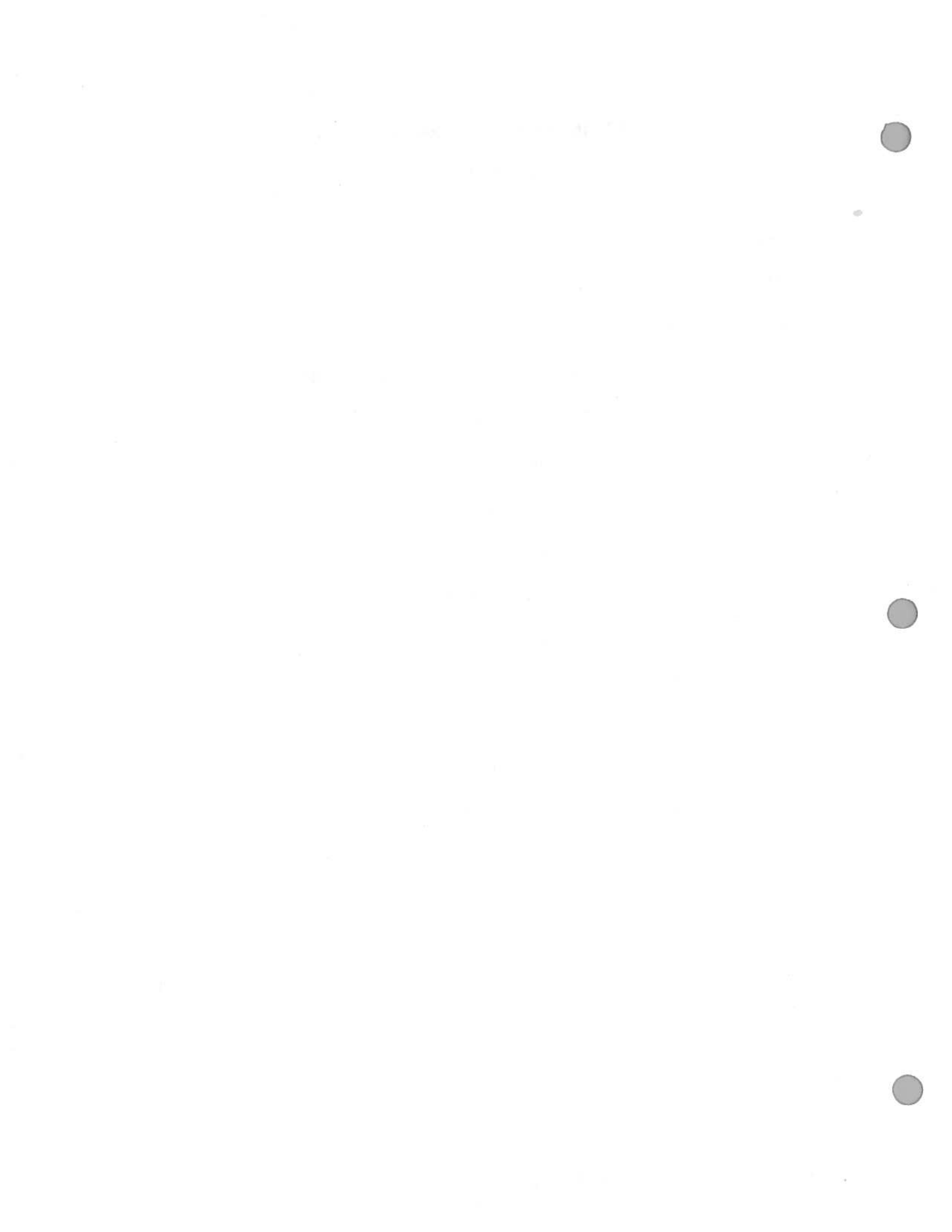


STORM DRAIN STANDARD PLANS

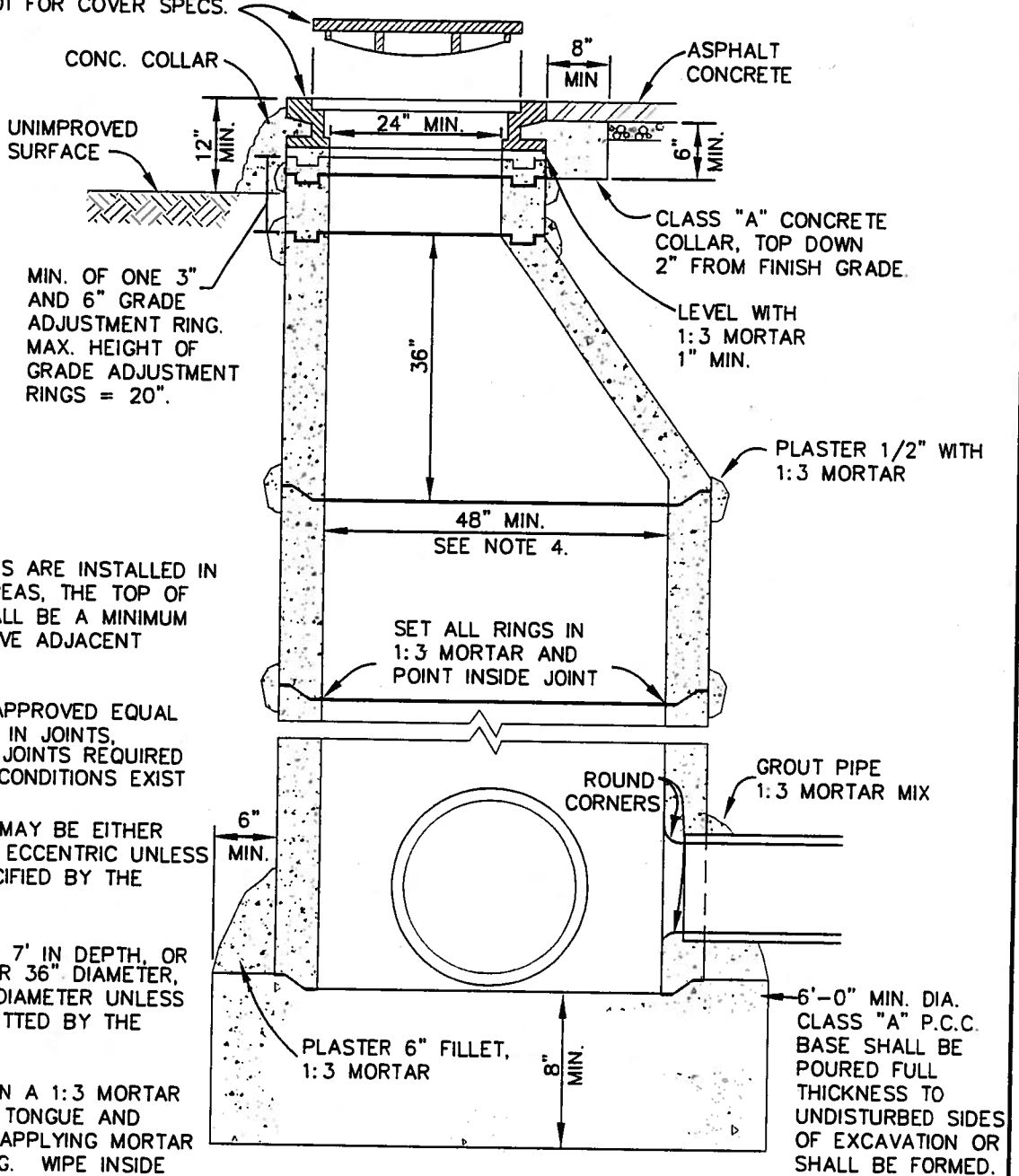
DESCRIPTION

400 SERIES – STORM DRAIN

400	Storm Drain Manhole
401	Standard Manhole Frame and Cover
402	Standard Pre-cast Concrete Storm Drain Manhole Reducer Slab
403	Pre-cast Catch Basin Hood
404	Curb Opening Catch Basin
405	Catch Basin for Pipes Larger Than 24"
406	Storm Drain Gallery
407	Temporary Redwood Box Field Inlet
408	Typical Storm Drain Outfall Detail
409	Sidewalk Drain
410	Sidewalk Cross Drain
411	Typical Lot Drainage
412	H.D.P.E. Trench Installation Detail



MANHOLE COVER AND FRAME
SOUTH BAY FOUNDRY SBF 1900, OR EQUAL.
SEE STD 401 FOR COVER SPECS.



NOTES:

1. WHEN MANHOLES ARE INSTALLED IN UNIMPROVED AREAS, THE TOP OF THE COVER SHALL BE A MINIMUM OF 1 FOOT ABOVE ADJACENT GRADE.
2. RAM-NEK OR APPROVED EQUAL SHALL BE USED IN JOINTS. PLASTERING OF JOINTS REQUIRED IF HIGH WATER CONDITIONS EXIST
3. CONE SECTION MAY BE EITHER CONCENTRIC OR ECCENTRIC UNLESS OTHERWISE SPECIFIED BY THE CITY ENGINEER.
4. MANHOLES OVER 7' IN DEPTH, OR WITH A PIPE OVER 36" DIAMETER, SHALL BE 5' IN DIAMETER UNLESS OTHERWISE PERMITTED BY THE CITY ENGINEER.
5. SET ALL RINGS IN A 1:3 MORTAR BED. WET BOTH TONGUE AND GROOVE BEFORE APPLYING MORTAR AND SETTING RING. WIPE INSIDE OF JOINTS SMOOTH AND PLASTER OUTSIDE OF JOINT WITH 1/2" LAYER OF MORTAR.
6. CONSTRUCT ALL FLOW CHANNELS OF PIPE WHEREVER POSSIBLE. AFTER BASE IS POURED, BREAK OUT TOP HALF OF PIPE FLUSH WITH INSIDE FACE OF M.H. WALL AND CONSTRUCT U-SHAPED CHANNEL. MAKE ELEVATION CHANGES GRADUALLY AND DIRECTIONAL CHANGES WITH SMOOTH CURVES. SET RING BASE IN MORTAR.
7. ALL SECTIONS OF MANHOLE MUST BE OF IDENTICAL MAKE AND MANUFACTURER.

FILE: BMAP-STD\CLOVER\STD\CLOV400-411.DWG



STORM DRAIN MANHOLE

STD. NO.
400

SCALE: NONE

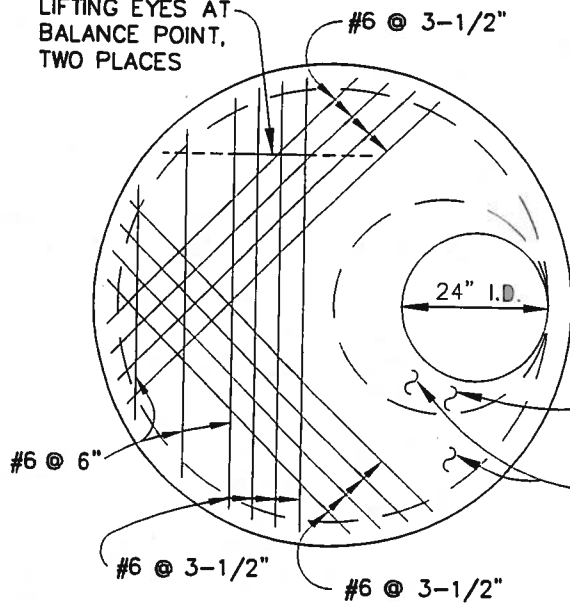
DRAWN: CLG

CHK: JHG

APPVD:

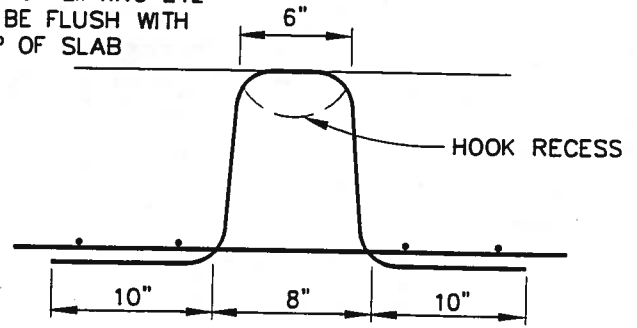
DATE: APR. 2004

LIFTING EYES AT
BALANCE POINT,
TWO PLACES



SLAB PLAN

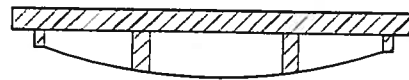
TOP OF LIFTING EYE
TO BE FLUSH WITH
TOP OF SLAB



LIFTING EYE DETAIL

4-#4 HOOPS AROUND
ACCESS OPENING

#2 @ 6" AROUND OPENING
SEE NOTE 2.

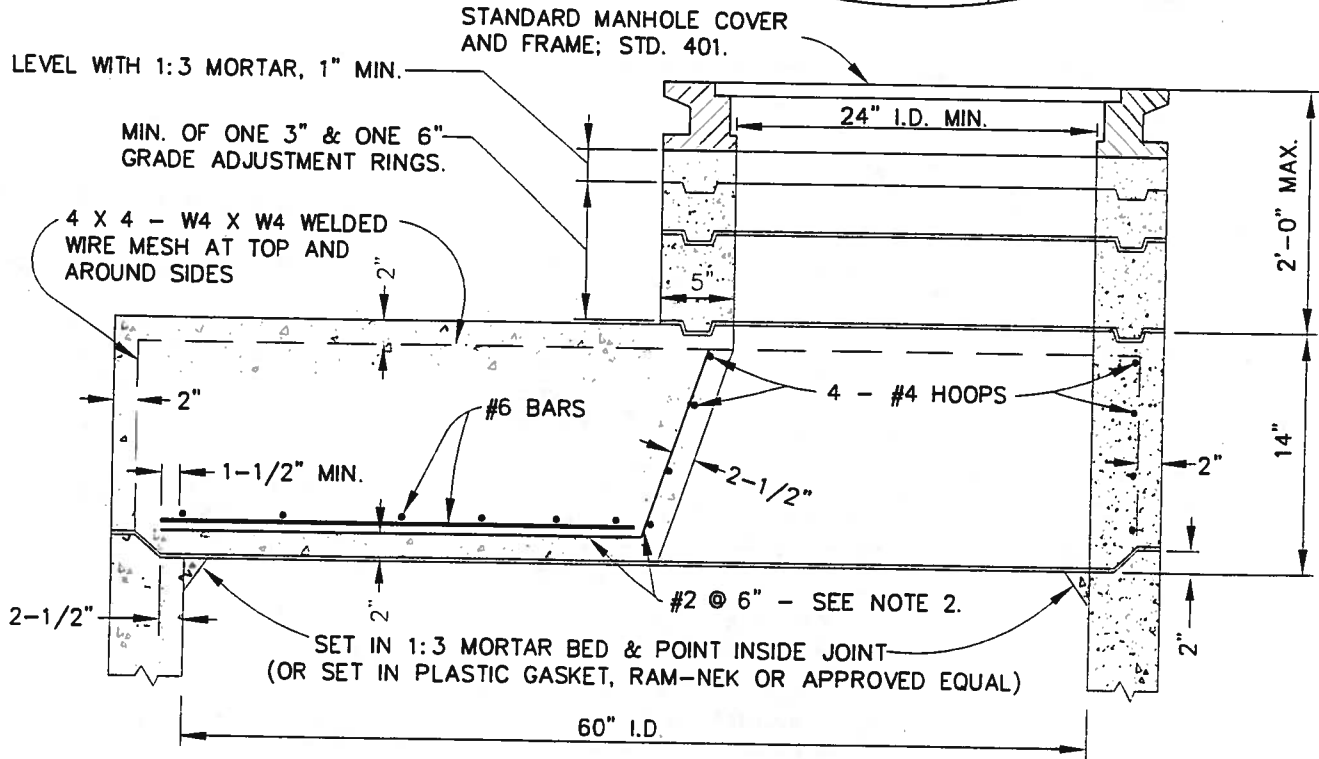


STANDARD MANHOLE COVER
AND FRAME; STD. 401.

LEVEL WITH 1:3 MORTAR, 1" MIN.

MIN. OF ONE 3" & ONE 6"
GRADE ADJUSTMENT RINGS.

4 X 4 - W4 X W4 WELDED
WIRE MESH AT TOP AND
AROUND SIDES



NOTES:

1. FOR DETAILS OF BASE AND BARREL SECTIONS, SEE CITY STD 400
2. #2 BARS BENT UP AND SPACED 6" O.C. AROUND 24" OPENING. HORIZONTAL LEGS TO FAN OUT EQUALLY SPACED, TO 2-1/2" CLEAR AT EDGE OF SLAB.

FILE: BMAP-STD\CLOVER\STD\CLOV400-411.DWG



**STANDARD PRECAST CONCRETE
STORM DRAIN MANHOLE
REDUCER SLAB**

STD. NO.
402

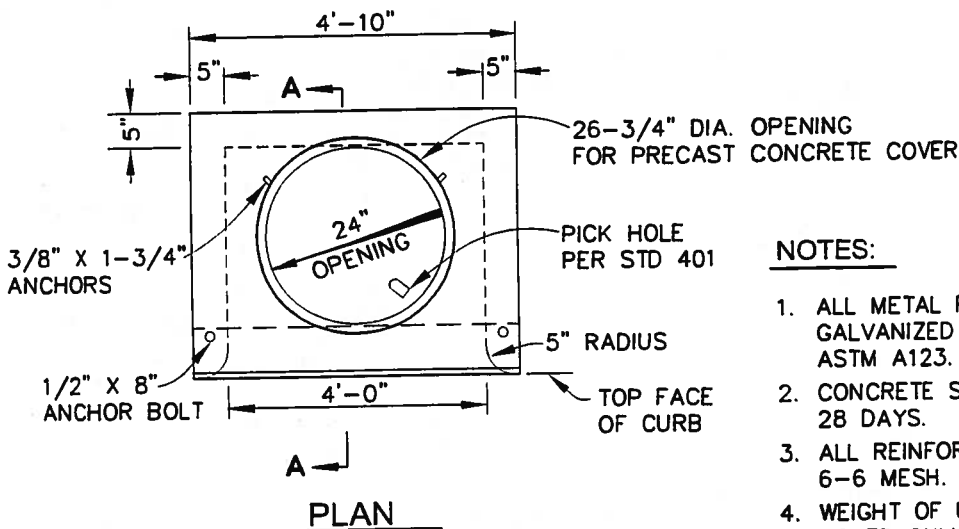
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DRAWN: CLG

CHK: JHG

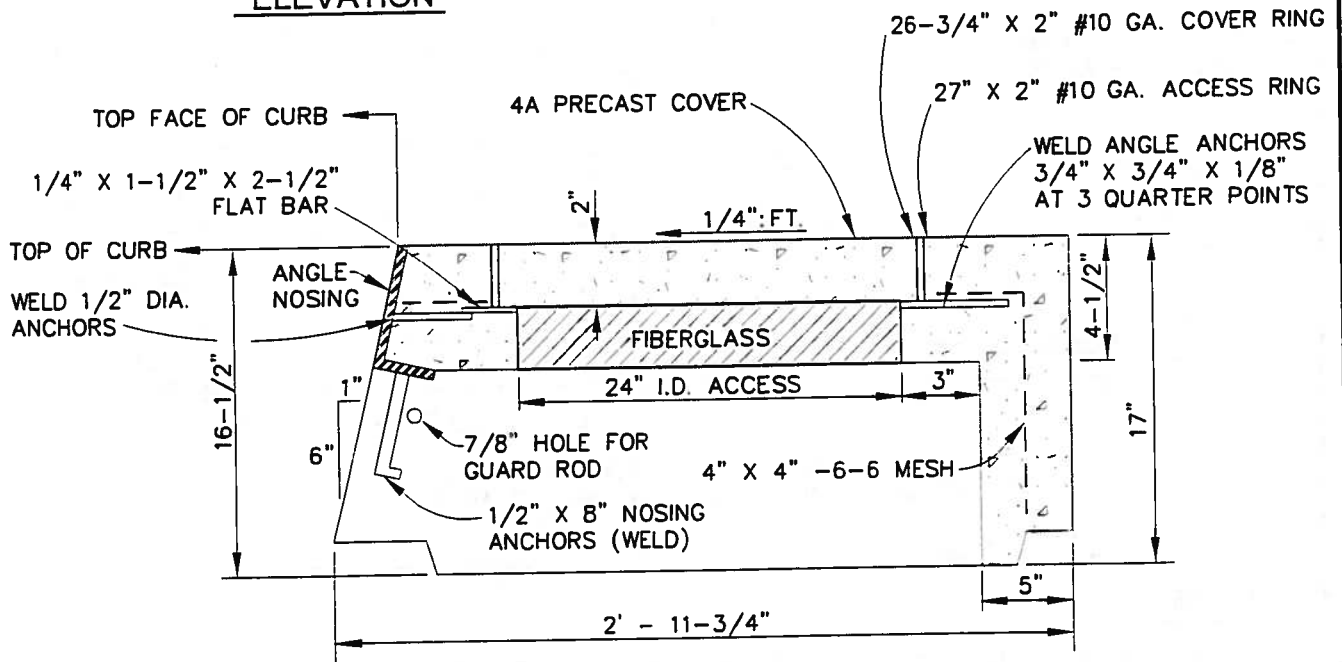
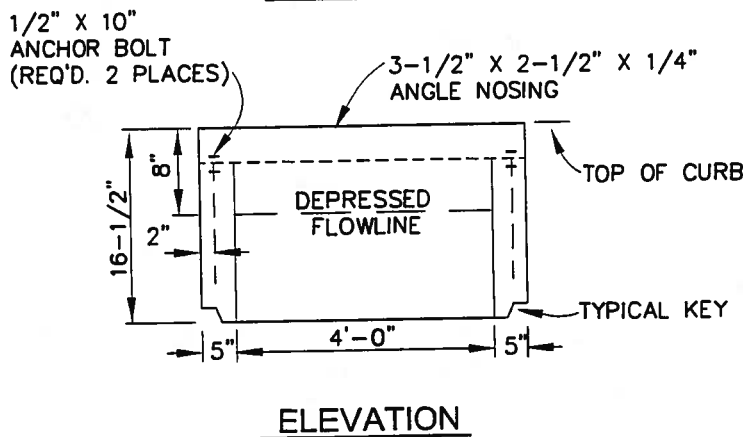
APPVD:

DATE: APR. 2004



NOTES:

1. ALL METAL PARTS SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION PER ASTM A123.
2. CONCRETE SHALL TEST 3000 PSI AT 28 DAYS.
3. ALL REINFORCING SHALL BE 4" X 4" - 6-6 MESH.
4. WEIGHT OF UNIT COMPLETE = 1500± LBS. COVER ONLY = 100± LBS.
5. 3/4" GALVANIZED STEEL GUARD ROD FOR OPENINGS IN EXCESS OF 9".
6. BASE MAY BE PRECAST OR CAST IN PLACE TO SUIT.



APPROVED ALTERNATES:
 CENTRAL PRE-CAST PRODUCTS MODEL 4AC
 PHEONIX PRECAST CONC. PRODUCTS MODEL P-2448-C

FILE: BMAP-STD\CLOVER\STD\CLOV400-411.DWG



**PRECAST
 CATCH BASIN HOOD**

STD. NO.
403

SCALE: NONE

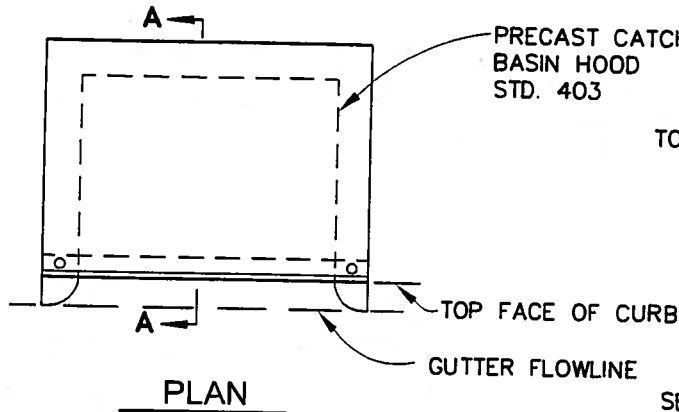
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CHK: JHG

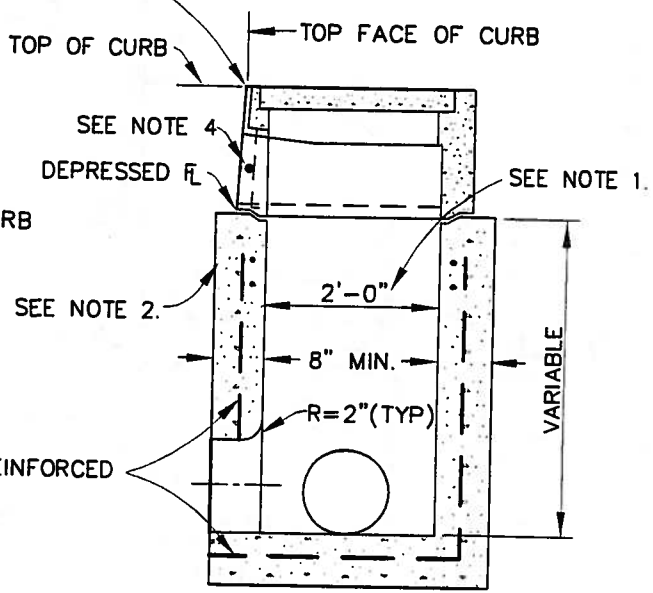
APPVD:

DATE: APR. 2004

BACK OF SIDEWALK

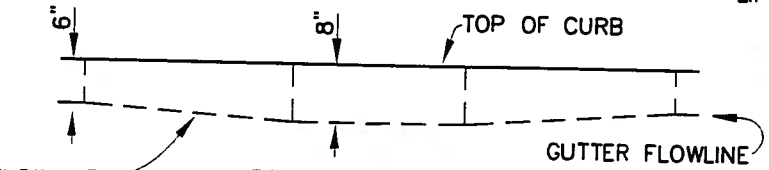
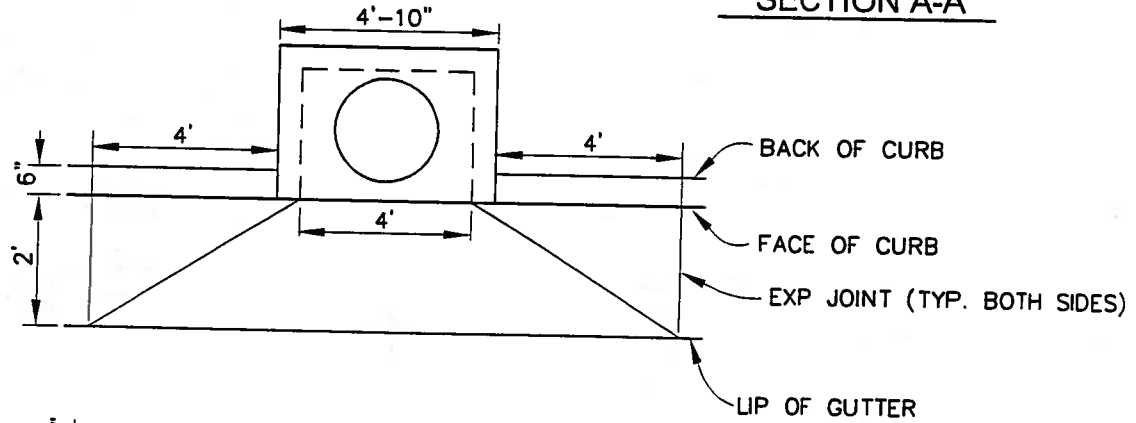


PLAN



SECTION A-A

WALLS AND FLOOR ARE REINFORCED WITH 4" x 4" W6-W6 WWF



CURB CONFORM

NOTES:

1. IF PIPE INTO OR OUT OF THE CATCH BASIN IS LARGER THAN 24", UNIT SHALL BE TAILOR MADE BY SUPPLIER, OR FIELD FABRICATED PER CITY STD. 405.
2. APPROVED ALTERNATES FOR CURB INLET BASE SECTIONS: CENTRAL PRE-CAST PRODUCTS BASE SECTION MODEL 4A; PHOENIX PRECAST CONC. PRODUCTS BASE SECTION MODEL D14.2
3. ALL HOOD, BASE, AND PIPE CONNECTIONS SHALL BE GROUTED.
4. 3/4" GALVANIZED STEEL GUARD ROD MUST BE INSTALLED AT CENTER OF OPENINGS IN EXCESS OF 9" INCHES IN LENGTH.

FILE: BMAP-SID\CLOVER\STD\CLOV400-411.DWG

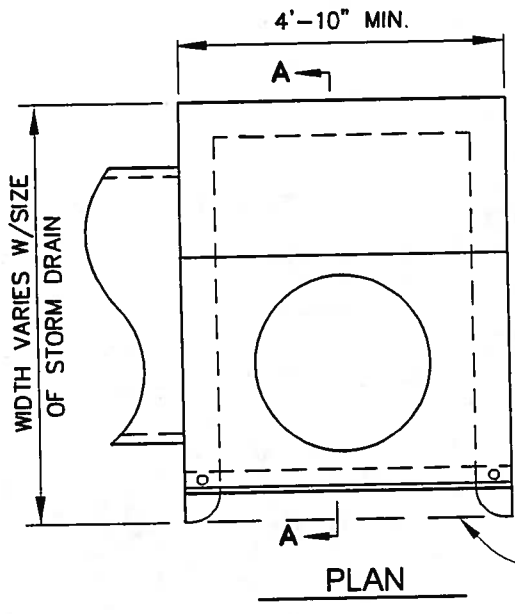


**CURB OPENING
CATCH BASINS**

STD. NO.
404

SCALE: NONE	DRAWN: LMM	CHK: JHG	APPVD:	DATE: APR. 2004
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FILE: BMAP-SID\CLOVER\STD\CLOV400-411.DWG



PRECAST CATCH
BASIN HOOD
STD. 403

TOP OF CURB

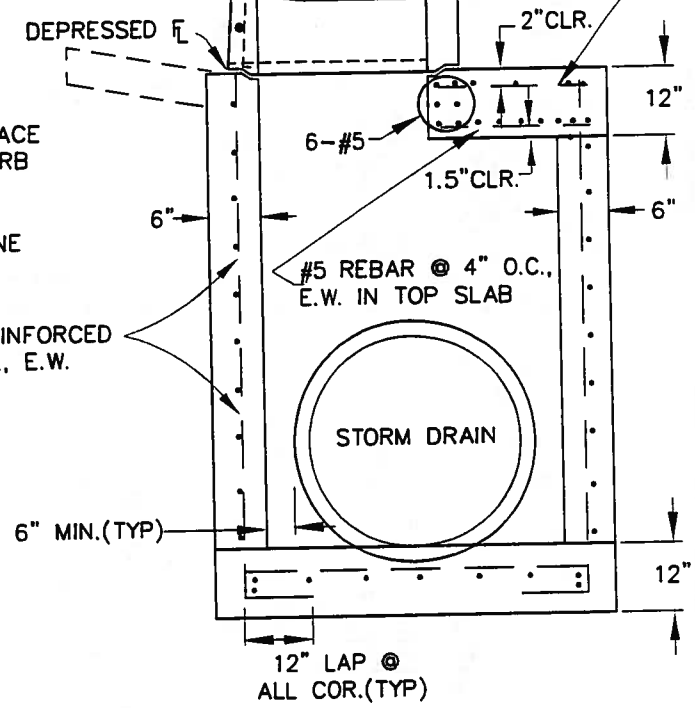
DEPRESSED FL

TOP FACE
OF CURB

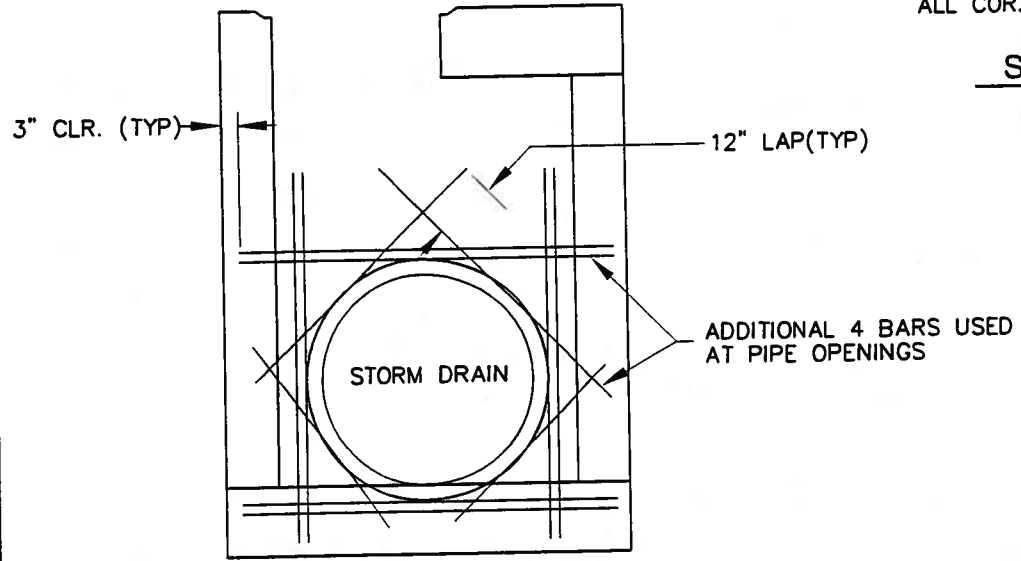
GUTTER
FLOWLINE

WALLS AND FLOOR ARE REINFORCED
WITH #5 REBAR @ 12" O.C., E.W.
AT CENTER OF WALL.

TEMPORARY REINFORCING
#3 REBAR @ 12" O.C., E.W.
(NOT SHOWN IN SECTION A-A)



SECTION A-A



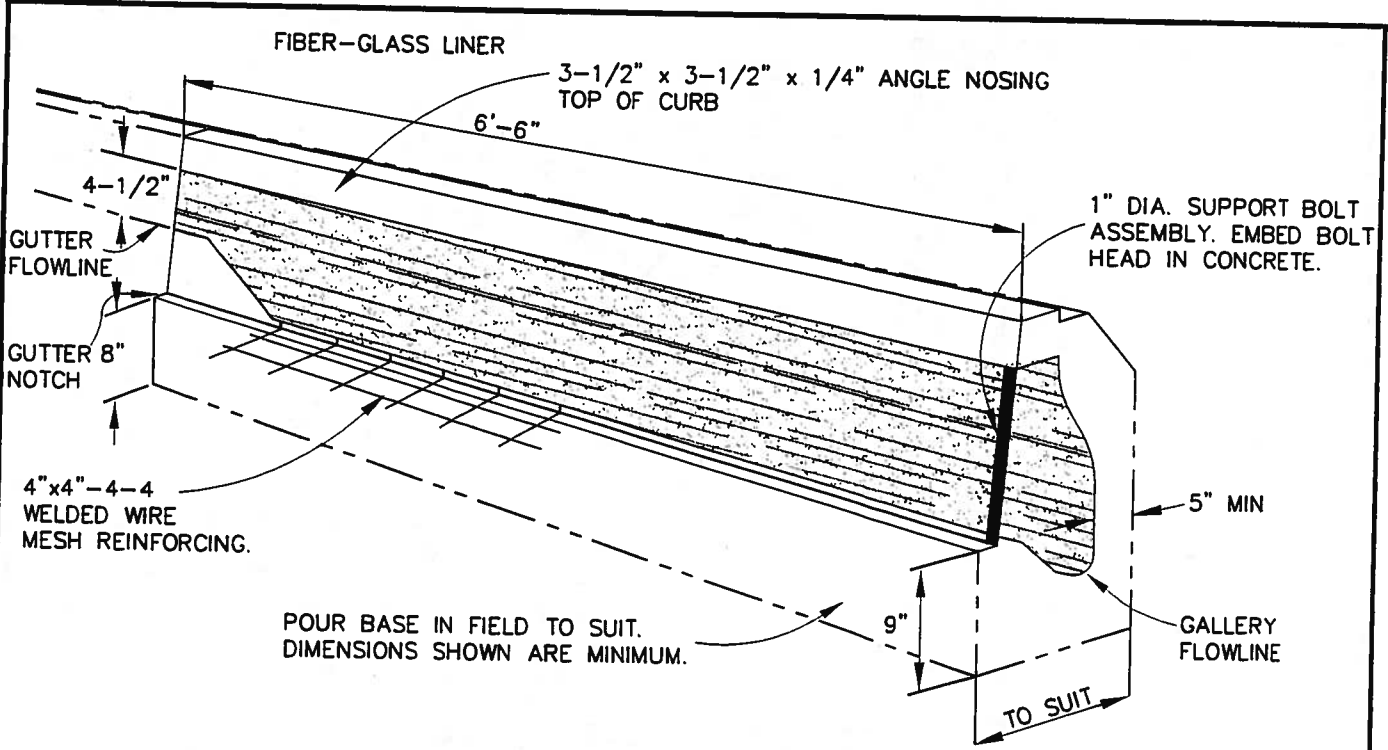
REINFORCING USED @ OPENINGS



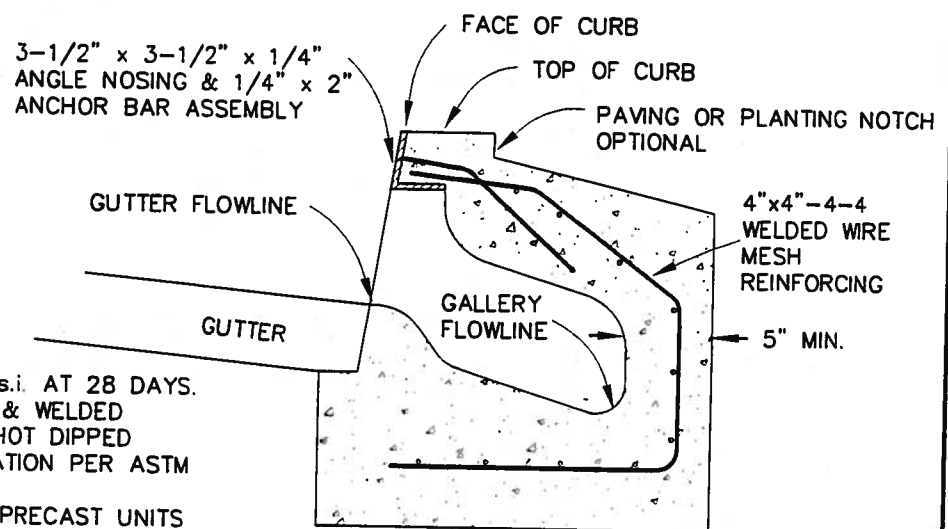
CATCH BASIN FOR PIPES LARGER THAN 24"

STD. NO.
405

SCALE: NONE | DRAWN: LMM | CHK: JHG | APPVD: | DATE: APR. 2004

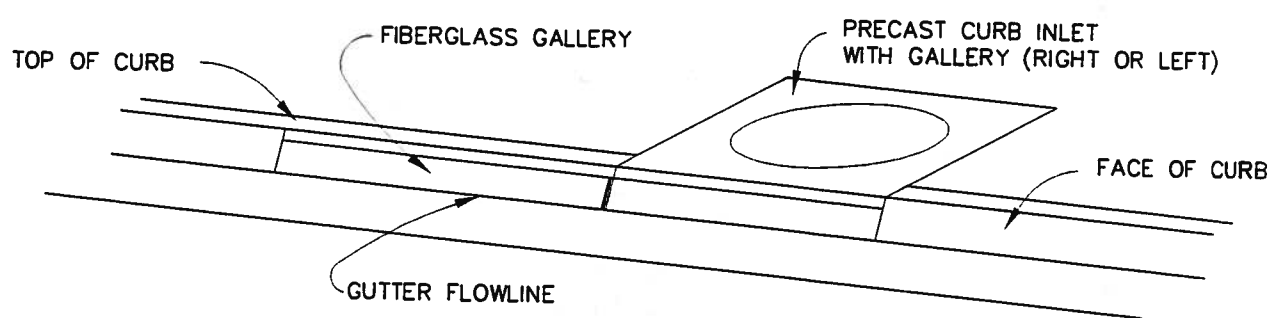


POUR BASE IN FIELD TO SUIT.
DIMENSIONS SHOWN ARE MINIMUM.



NOTES:

1. CONCRETE SHALL BE 3000p.s.i. AT 28 DAYS.
2. NOSING ASSEMBLY (ANGLED & WELDED ANCHOR BARS) SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION PER ASTM SPEC. A123-59.
3. EITHER CAST-IN-PLACE OR PRECAST UNITS ARE ACCEPTABLE.



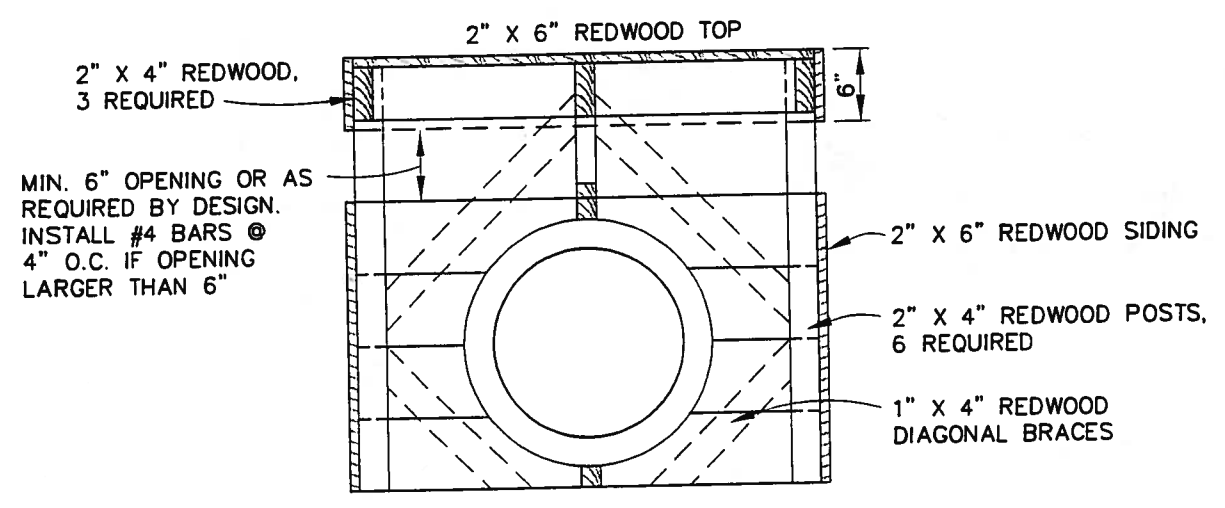
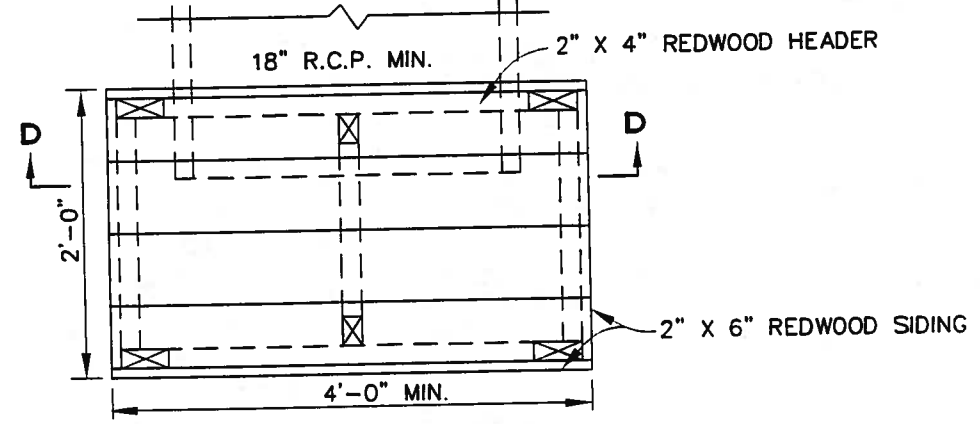
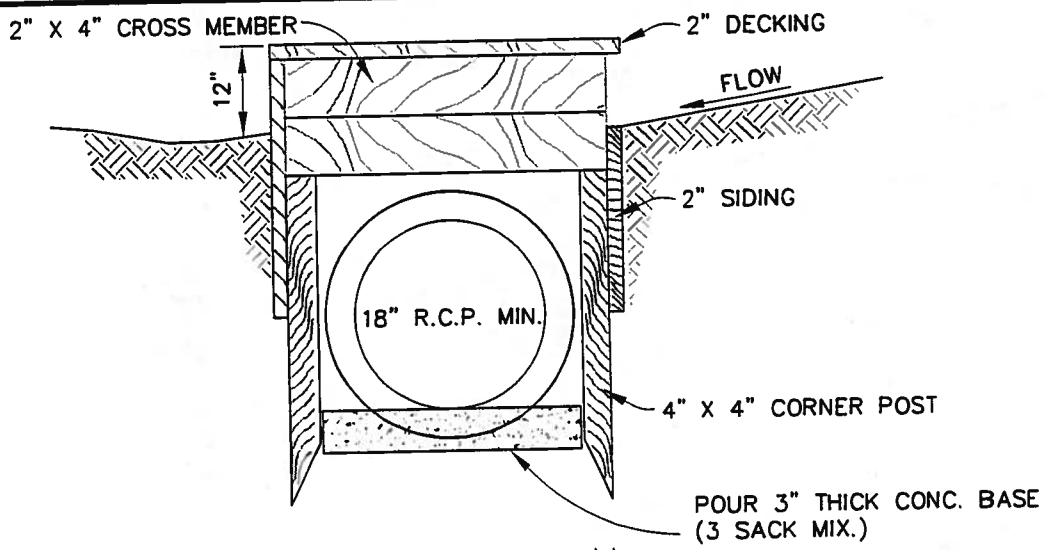
FILE: BMAP-STD\CLOVER\STD\CLOV400-411.DWG



STORM DRAIN GALLERY

STD. NO.
406

SCALE: NONE | DRAWN: CLG | CHK: JHG | APPVD: | DATE: APR. 2004

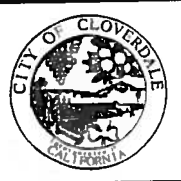


SECTION D-D

NOTES:

1. ALL WOOD SHALL BE CONSTRUCTION HEART REDWOOD OR BETTER.
2. HOT DIPPED GALVANIZED NAILS SHALL BE USED THROUGHOUT.
3. THIS DETAIL IS TO BE USED IF THE DURATION OF USE IS LESS THAN 2 YEARS. USE A CONCRETE STRUCTURE IF LONGER DURATION.

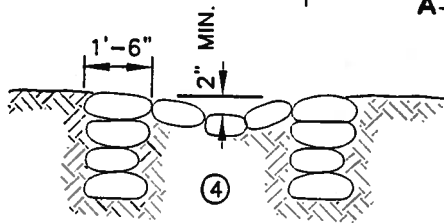
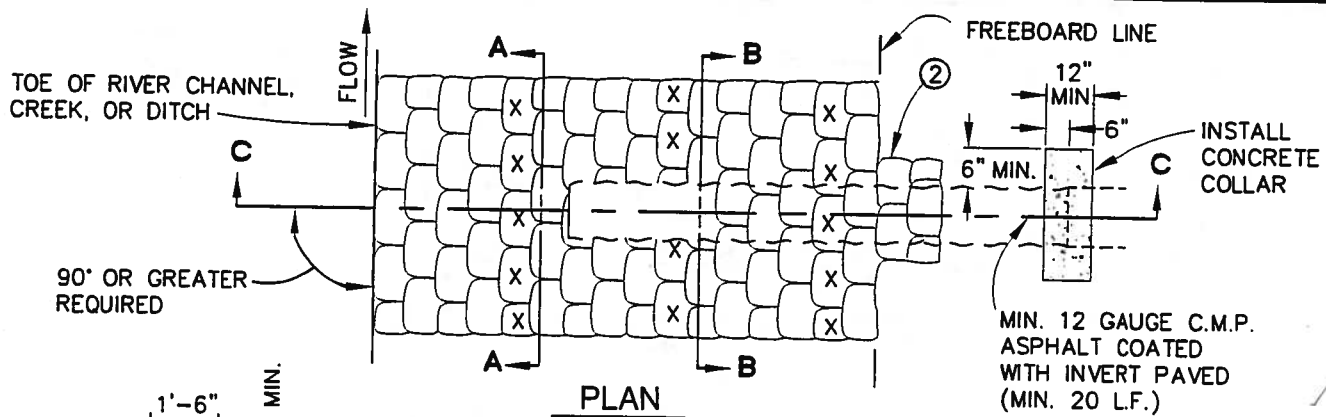
FILE: BMAP-STD\CLOVER\STD\CLOV400-411.DWG



TEMPORARY REDWOOD BOX
FIELD INLET

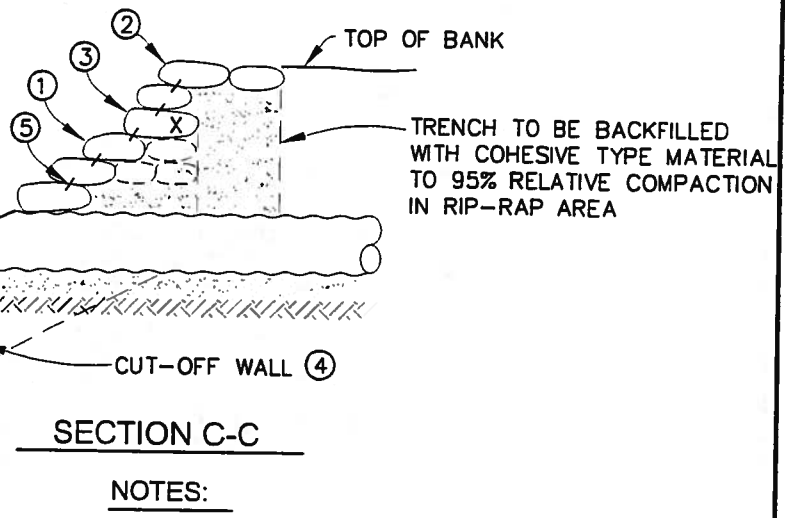
STD. NO.
407

SCALE: NONE | DRAWN: CLG | CHK: JHG | APPVD: | DATE: APR. 2004

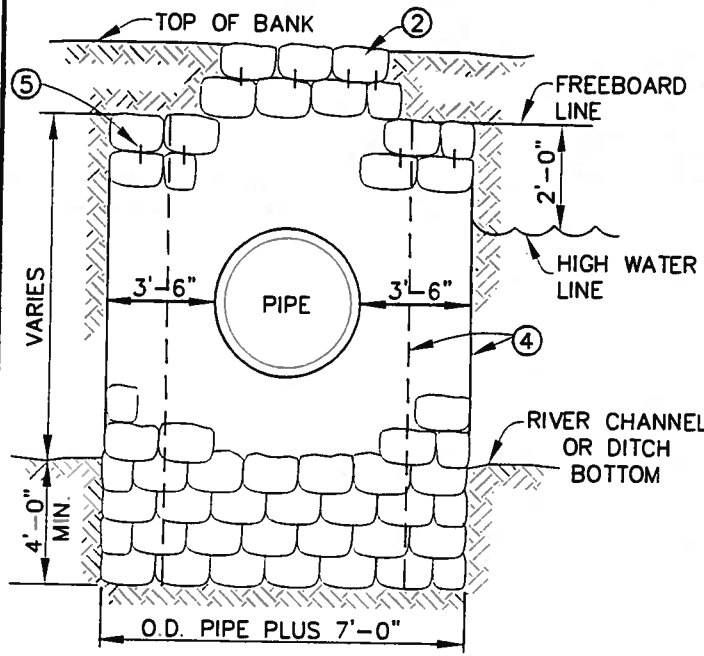


SECTION A-A

SLOPE 1.5:1 MAX.
OR EQUAL TO EX.
SLOPE IF LESS



SECTION C-C



SECTION B-B

NOTES:

1. FACE OF RIP-RAP TO BE COINCIDENT WITH EXISTING (OR FUTURE DESIGN) SIDE SLOPE OF CHANNEL.
2. CARRY RIP-RAP TO TOP OF BANK IN TRENCH EXCAVATION ABOVE FREEBOARD.
3. SACK CONCRETE RIP-RAP PLACED ON UNDISTURBED SOIL. ANY OVER EXCAVATION MUST BE FILLED WITH SACKS, NO EARTH BACKFILL WILL BE PERMITTED. (EVERY FIFTH COURSE TO BE A HEADER COURSE. ⊗)
4. INSTALL CUT-OFF WALL UPSTREAM & DOWN-STREAM TO AN ELEVATION WHICH IS EQUAL TO THE FREEBOARD ELEVATION OR TOP OF PIPE, WHICH EVER IS HIGHER, & ACROSS THE BOTTOM WITH 4.0 MINIMUM DEPTH.
5. IN ALL TOP COURSES AND THROUGHOUT, IF SIDE SLOPE IS STEEPER THAN 1:1 AND/OR HIGHER THAN 10 FEET, DRIVE ONE #4 REINFORCING BAR 18\"/>
- 6. OUTFALLS TO BE APPROVED BY FISH AND GAME AND FLOOD CONTROL AS APPROPRIATE.

FILE: BMAP-S1D\CLOVER\STD\CLOV400-411.DWG

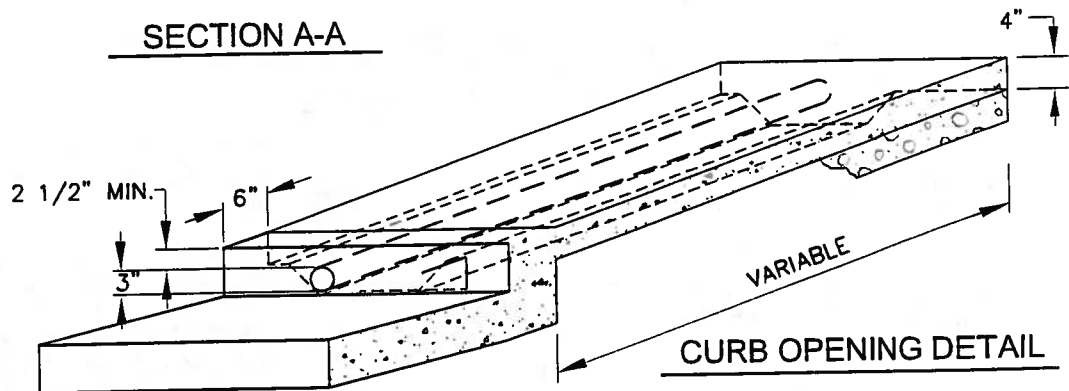
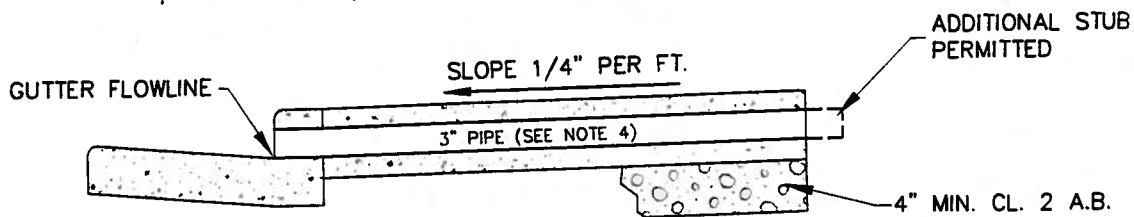
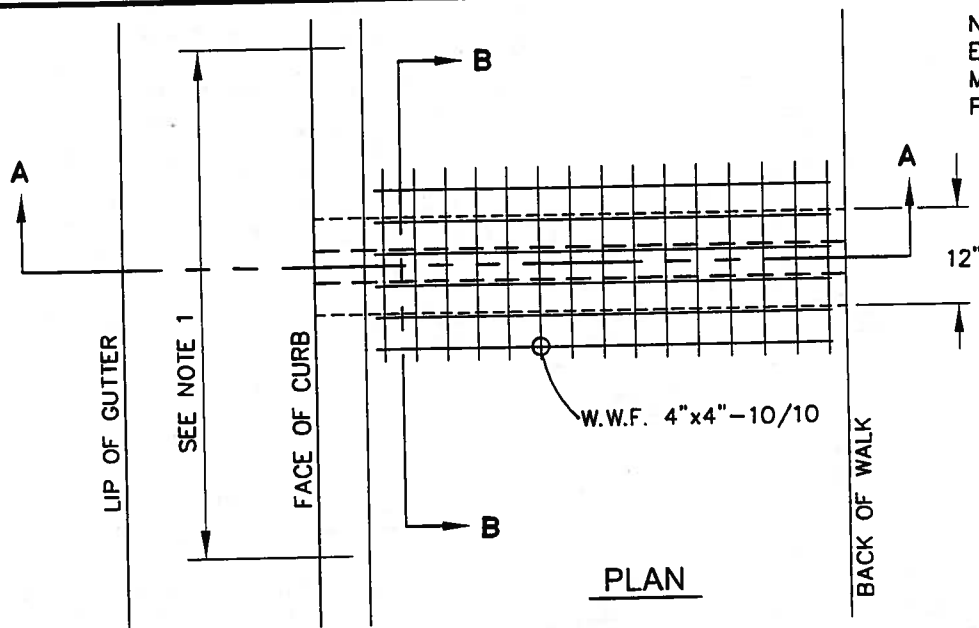


TYPICAL STORM DRAIN OUTFALL DETAIL

STD. NO.
408

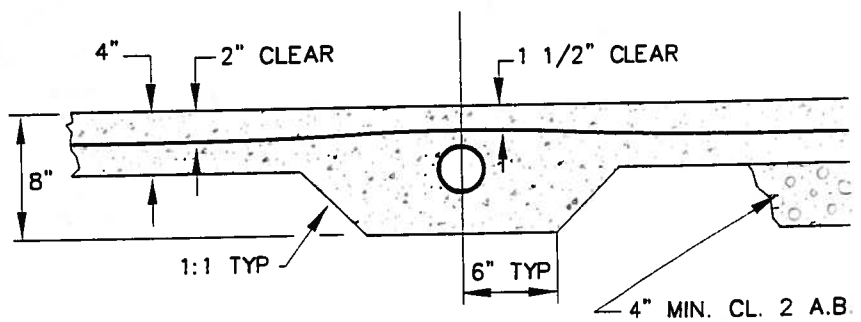
SCALE: NONE	DRAWN: CLG	CHK: JHG	APPVD:
DATE: APR. 2004			

NOTE: IF SIDEWALK IS EXISTING, A 12" SECTION MAY BE SAWCUT TO PLACE 3" P.V.C. & W.W.F.



NOTES:

1. WIRE MESH SHALL BE 2' WIDE. LENGTH SHALL EQUAL SIDEWALK WIDTH MINUS 4". IF SIDEWALK IS EXISTING, SEE NOTE ABOVE.
2. ON-SITE DRAINAGE AND LOCATION OF CURB OUTLETS SHALL BE BY THE OWNER TO THE SATISFACTION OF THE CITY ENGINEER.
3. DRAIN PIPE SHALL BE INSTALLED SO THAT TOP OF PIPE IS 2-1/2" MIN. BELOW FINISH GRADE AT BACK OF SIDEWALK.
4. SIDEWALK DRAIN TO BE 3" SCH. 40, HEAVY WALL, RIDGID POLYVINYL CHLORIDE PIPE OR APPROVED SUBSTITUTE.



FILE: BMAP-STD\CLOVER\STD\CLOV400-411.DWG



SIDEWALK DRAIN

STD. NO.

409

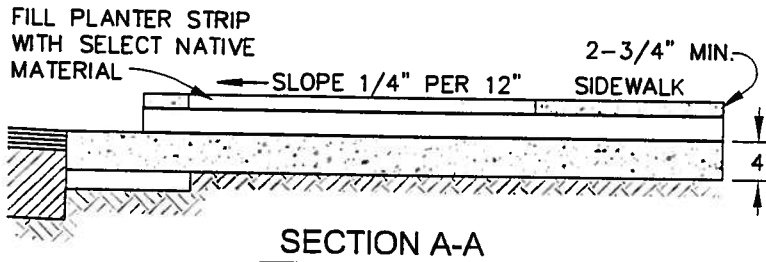
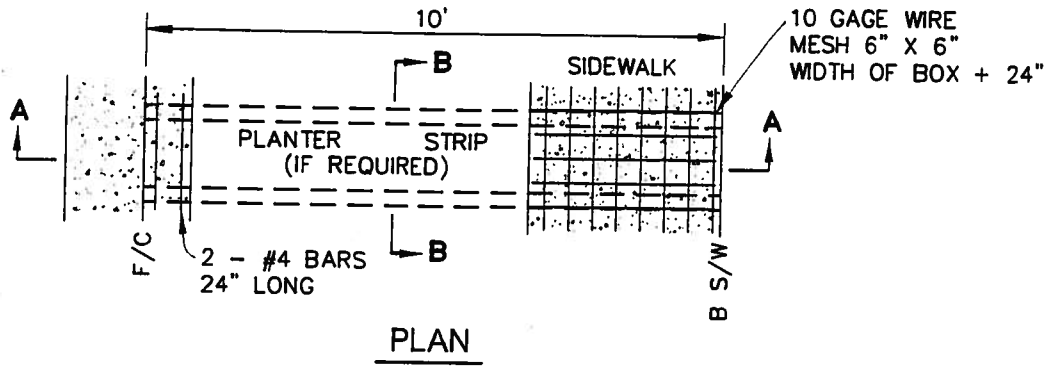
SCALE: NONE

DRAWN: LMM

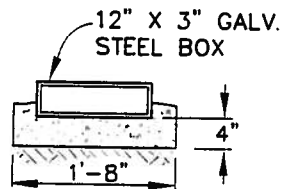
CHK: JHG

APPVD:

DATE: APR. 2004



SECTION A-A



SECTION B-B

NOTES:

1. WITH APPROVAL OF THE CITY ENGINEER, WIDTH OF BOX MAY VARY FROM 6" TO 12".
2. GALVANIZED STEEL TO BE 1/4" THICK.
3. ALL CONCRETE SHALL BE CLASS "A" (6 SACKS PER CUBIC YARD).

FILE: BMAP-STD\CLOVER\STD\CLOV400-411.DWG



SIDEWALK CROSS DRAIN

STD. NO.

410

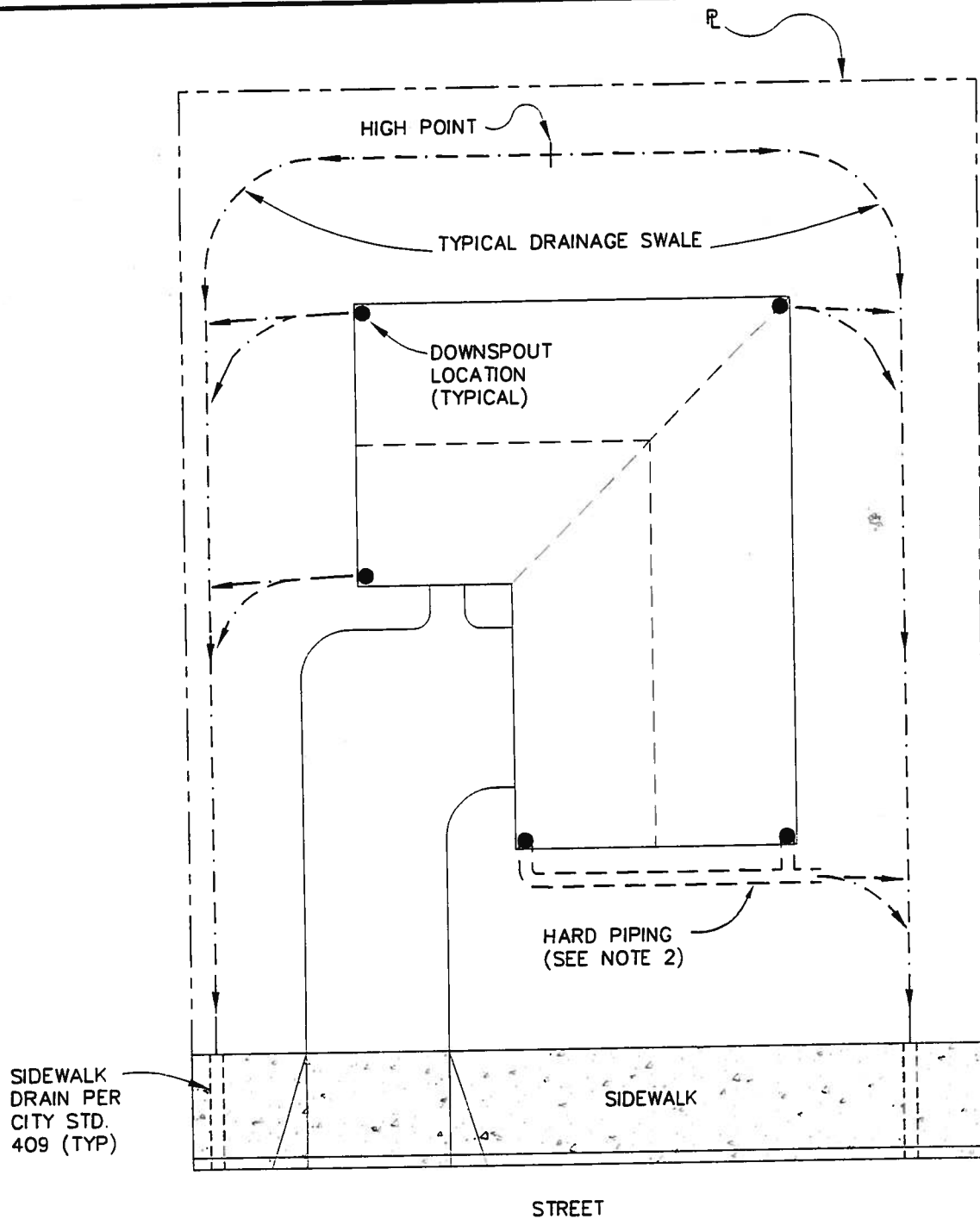
SCALE: NONE

DRAWN: LMM

CHK: JHG

APPVD:

DATE: APR. 2004



NOTES:

1. ALL ROOF DRAINAGE MUST BE ROUTED FROM EACH DOWNSPOUT THROUGH SURFACE SWALES TO SIDEWALK DRAIN OR OTHER APPROVED DRAINAGE STRUCTURE.
2. HARD PIPING SHALL BE FLEXIBLE A.D.S. PIPE WITH POSITIVE DRAINAGE TO SWALES, AS APPROVED BY THE CITY ENGINEER OR BUILDING OFFICIAL.
3. ALL CONCENTRATED DRAINAGE FROM A PARCEL MUST BE INTERCEPTED INTO AN UNDERGROUND SYSTEM PRIOR TO CROSSING SIDEWALKS.
4. ALL HARD PIPING SHALL BE BURIED.

FILE: BMAP - STD\CLOVER\STD\CLOV400-412.DWG



TYPICAL LOT DRAINAGE

STD. NO.

411

SCALE: NONE | DRAWN: LMM | CHK: JHG | APPVD:

DATE: MAY 2004

NOTES:

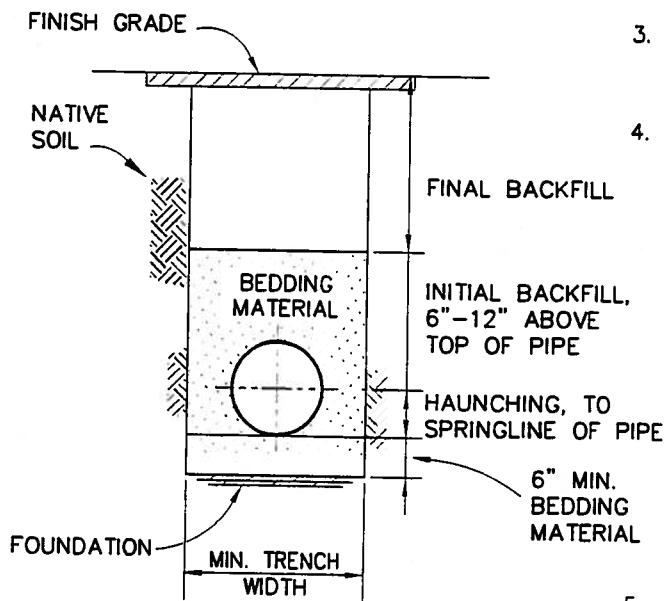
1. **FOUNDATION:** WHERE THE TRENCH BOTTOM IS UNSTABLE, THE CONTRACTOR SHALL EXCAVATE TO A DEPTH REQUIRED BY THE ENGINEER AND REPLACE WITH A FOUNDATION OF CLASS I OR II MATERIAL AS DEFINED IN ASTM D2321, "STANDARD PRACTICE FOR INSTALLATION OF THERMOPLASTIC PIPE FOR SEWERS AND OTHER GRAVITY-FLOW APPLICATIONS," LATEST EDITION; AS AN ALTERNATIVE AND AT THE DISCRETION OF THE ENGINEER, THE TRENCH BOTTOM MAY BE STABILIZED USING A WOVEN GEOTEXTILE FABRIC.
2. **BEDDING:** SUITABLE MATERIAL SHALL BE CLASS I, II, OR III, AND INSTALLED AS REQUIRED IN ASTM D2321, LATEST EDITION.
3. **HAULING AND INITIAL BACKFILL:** SUITABLE MATERIAL SHALL BE CLASS I, II, OR III, AND INSTALLED AS REQUIRED IN ASTM D2321, LATEST EDITION.
4. UNLESS OTHERWISE SPECIFIED BY THE ENGINEER, MINIMUM TRENCH WIDTHS SHALL BE AS FOLLOWS:

NOMINAL ϕ IN INCHES	MIN. RECOMMENDED TRENCH WIDTH
4"	21"
6"	23"
8"	25"
10"	28"
12"	31"
15"	34"
18"	39"
24"	48"
30"	66"
36"	78"
42"	83"
48"	89"
60"	102"

5. **MINIMUM COVER:** MINIMUM RECOMMENDED DEPTHS OF COVER FOR VARIOUS LIVE LOADING CONDITIONS ARE SUMMARIZED IN THE FOLLOWING TABLE. UNLESS OTHERWISE NOTED, ALL DIMENSIONS ARE TAKEN FROM THE TOP OF PIPE TO THE GROUND SURFACE.

SURFACE LIVE LOADING CONDITION	MINIMUM RECOMMENDED COVER IN INCHES
H25 (FLEXIBLE PAVEMENT)	24"
H25 (RIGID PAVEMENT)	24"
E80 RAILWAY	24"
HEAVY CONSTRUCTION	48"

TOP OF PIPE TO BOTTOM OF BITUMINUS PAVEMENT SECTION.



TYPICAL TRENCH CROSS-SECTION
(N.T.S.)

FILE: BMAP--STD\CLOVER\STD\CLOV400--412.DWG



HDPE TRENCH INSTALLATION DETAIL

STD. NO.
412

SCALE: NONE

DRAWN: CLG

CHK: JHG

APPVD:

DATE: APR. 2004

